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MARCENE REPORT

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REPORT



PROJECT No	FR1636
PROJECT TITLE	BARRY – BIOMASS UK Number 2 LTD
CLIENT	GALLIFORD TRY

DISCIPLINE	CIVIL / STRUCTURAL
DOCUMENT TITLE	SURFACE WATER DRAINAGE DESIGN
DOCUMENT NUMBER	BARRY_01_REP_01_20063

			M. Ser.	Kc	
А	HA	12/07/17	MS	GB	FOR INFORMATION
ISSUE	CREATED BY	DATE	CHECKED BY	APPROVED BY	REASON FOR ISSUE



REPORT

CHANGE TRACKING LOG						
Revision	Date	Description				
А	12/07/2017	Firstissue				



REPORT	
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REPORT

PART A – INTRODUCTION

A1- DESCRITPION

This report covers the design of the surface water drainage network for the Barry EFW project.

Drawings are attached in the Appendix D highlighting layouts of the drainage network.

A2- STRUCTURAL FORM

The surface water drainage network utilises a Tubosider attenuation tank for storage capacity and ACO drains.

A restricted discharge for the surface water drainage of 3 l/s has been imposed by PCML.

A3- SURFACE WATER DRAINAGE DESIGN PARAMETERS

Design of the network is carried out using MicroDrainage System 1 to set up the network and Simulation to run the storm events through the network.

The following parameters were used to assess the new surface water drainage network on the site:

- 1:1 year storm event (+20% for future anticipated climate change BS EN 752 Cl. 8.4.3.3) to check the manholes are not surcharged where possible (BS EN 752 NA.4.1.2 & 4.2.3.3).
- Surcharging of individual manholes is considered acceptable, in a 1:1 year storm event, provided that the surcharging does not pose a flood risk. A manhole will pose a flood risk when the surcharge level is within 300mm of the manhole cover level.
- 1:30 year storm event (+20% for future anticipated climate change BS EN 652 Cl. 8.4.3.3) to check the network does not flood (BS EN 752 NA.4.1.2).
- 1:100 year storm event (+20% for future anticipated climate change BS EN 752 CI. 8.4.3.3) to check that any flood that occurs is contained on site.
- Various storm durations between 15 minutes and 10080 minutes (7 days) are analysed to find the critical storm for design.
- An infiltration rate of 0% was used to all concrete/tarmac hardstanding areas.
- Due to the levels of the site the discharge will be pumped at a restricted discharge rate imposed by PCML (3 l/s).
- Pipes are to be laid generally with a minimum 1:300 fall, with a steeper gradient where required to avoid clashes with other services.
- Surface water from the hardstanding areas and the roof areas are to be laid to gravity falls into a Tubosider Storm Water Retention tank.



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REPORT

A4- ACO DRAINAGE DESIGN PARAMETERS

ACO drainage design is carried out using ACO's online design programme at www.acodesign.co.uk.

The following parameters were used to design the kerb drains and slot drains on the site:

- 1:30 year storm event to check the system has the required flow capacity.
- An infiltration rate of 0% was used to all concrete/tarmac hardstanding areas.
- All kerb drains and slot drains are assumed to have a level invert, with stepped transitions between different sized sections (where applicable).

A5- DESIGN DECISIONS TAKEN BY OTHERS

• PCML discharge consent flow rate 3 l/s.

A6- DESIGN ASSUMPTIONS

- No allowance has been made for run-off from grass verges or surrounding landscaping. These areas are assumed to have a 100% infiltration rate.
- Pipes in the network are assumed to have a roughness coefficient of 0.6mm.

A7-DESIGNEXCLUSIONS

- Temporary works.
- Foul sewer network.
- Rainwater harvesting tanks.



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REPORT

A8- REFERENCES

Document Reference

Title

[1] BS EN 752:2008[2] BARRY_01_DWG_01_20131 to 20136[3] TUB/15257

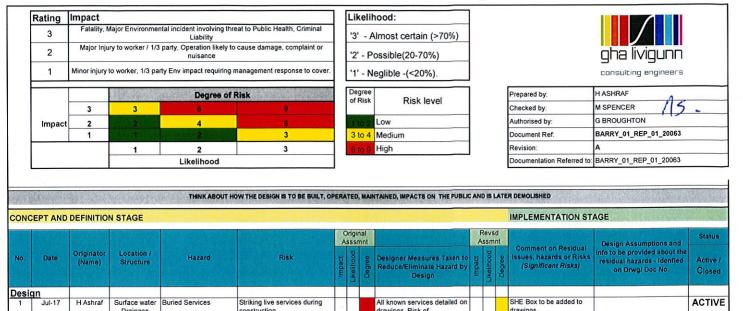
Drain and sewer systems outside buildings Site services Gas & schedule Storm Water Retention Tank for Barry Biomass



REPORT

APPENDIX A – DESIGN RISK ASSESSMENT

DESIGN RISK MANAGEMENT LOG PROJECT: Surface Water Drainage



Desig	111									_		_		
	Jul-17	H Ashraf	Surface water Drainage	Buried Services	Striking live services during construction.	3	2	6	All known services detailed on drawings. Risk of encountering unknown services shown on drawings. Site investigation including CAT SCAN to locate all chartered and unchartered services. Notes added to drawing reagration cisk	3	1	3	SHE Box to be added to drawings.	ACTIVE
2	Jul-17	H Ashraf	Surface water Drainage		Damage to the kerb drainage system	3	2	6	ACO KerbDrains to be installed in accordance with the manufacturer's installation detail, which includes a mass concrete surround.	3	1	3	Notes and construction details to be added to drawings.	ACTIVE
3	Jul-17	H Ashraf	Surface water Drainage		Damage to the slot drainage system	3	2	6	ACO Q-Max slot drainage system to be installed in accordance with the manufacturer's installation detail, which includes a mass concrete surround. System to be provided with D400 access covers.	3	1	3	Notes and construction details to be added to drawings.	ACTIVE
Oper	ation / M	aintenand	e Period											

Impacts on the Public

Demolition

Designers have to consider ALL HAZARDS and do what is reasonable to eliminate if feasible, or reduce risks where hazards remain. But, when it comes to PASSING ON INFORMATION, Designers do not need to mention every hazard or assumption, but they MUST POINT OUT SIGNIFICANT RISKS. These are not necessarily those that result in greatest risk, but those that are:- a) not likely to be obvious to a competent Designer or Contractor, b) unusual, or c) likely to be difficult to manage effectively. Note 1: The Appointed Temporary Works Coordinator must ensure that any temporary works designs are undertaken by a suitably competent person and be risk assessed.



REPORT

APPENDIX B – MICRODRAINAGE OUTPUT



CALCULATIONS

Client GALLIFO	RO TRY		Job no. FAIG3C	Sheet of
Project BARRY	EFW		Calcs by HA	Date 12(07)1
Section PRATINA	se design	Rev. A	Checked by	Date
Reference				
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George Hutchison Assoc	iates Limited	Page 1
The Studio	BARRY EFW	
51 Brookfield Road	SURFACE DRAINAGE	
Cheadle <u>SK8</u> 1ES		
Date 06/07/2017	Designed By HA	DPATTAG
File MODEL HA1.SUM	Checked By M4	
Micro Drainage	Simulation W.11.2	

<u>On-Line Controls (Pump)</u>

US/PN	Volume	Ctrl	Invert	Headloss	Flow
	(m³)	MH Name	(m)	(m)	(1/s)
1.006	606.202	20	5.300	0.2 0.4 0.6 1.0 1.4 1.8 2.2 2.6 3.0 3.4 3.8 4.2	3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0

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George Hutchison Associat	Page 2	
The Studio	BARRY EFW	
51 Brookfield Road	SURFACE DRAINAGE	IN MERO ~ ~
Cheadle SK8 1ES		
Date 06/07/2017	Designed By HA	DETERT
File MODEL HA1.SUM	Checked By MS	
Micro Drainage	Simulation W.11.2	

<u>Network Details</u>

 \star - Indicates pipe has been modified outside of WinDes's Storm/Foul & Schedules

PN	Length (m)	Fall (m)	Slope (1:x)	Area (ha)	T.E. (mins)	Rain Pro	k (mm)	Hyd Sect	Dia (mm)
1.000	31.10	0.104	299,0	0.124	5.00	1	0.600	0	225
1.001	16.20	0.054	300.0	0.064	0.00	1	0.600	0	300
1.002	28.15	0.094	299.5	0.058	0.00	1	0.600	0	300
2.000	29.20	0.097	301.0	0.058	5.00	1	0.600	0	225
2.001	18.58	0.062	299.7	0.016	0.00	1	0.600	0	225
1.003	9.61	0.048	200.2	0.000	0.00	1	0.600	0	300
3.000	12.88	0.043	299.5	0.008	5.00	1	0.600	0	150
3.001	19.00	0.063	301.6	0.000	0.00	1	0.600	0	150
3.002	10.92	0.037	295.1	0.036	0.00	1	0.600	0	225
3.003	11.06	0.037	298.9	0.027	0.00	1	0.600	0	300
3.004	29.60	0.100	296.0	0.058	0.00	1	0.600	0	300
3.005	25.46	0.090	282.9	0.047	0.00	1	0.600	0	300
4.000	19.26	0.064	300.9	0.052	5.00	1	0.600	0	225
3.006	27.21	0.091	299.0	0.000	0.00	1	0.600	0	300
3.007	11.08	0.939	11.8	0.035	0.00	1	0.600	0	300
1.004	31.60	0.131	240.5	0.116	0.00	1	0.600	0	600
5.000	8.79	0.051	171.0	0.036	5.00	1	0.600	0	225
1.005	10.00	0.239	41.9	0.000	0.00	1	0.600	0	600
1.006	140.00	0.050	2800.0	0.000	0.00	1	0.600	0	2400
1.007	5.00	0.300	16.7	0.000	0.00	1	0.600	0	150

PN	USMH No.		US/CL (m)	US/IL (m)	US C.Depth (m)	DS/CL (m)	DS/IL (m)	DS C.Depth (m)	Ctrl No	US/MH (mm)
1.000		SW06	8.350	6,660	1.465	8,440	6.556	1,659		1200
1.001		SW05	8.440	6.556	1.584	8 477	6.502	1.675		1200
1.002		SW04	8.477	6.502	1.675	8.911	6.408	2.203		1200
2.000		SW09	8.667	7.100	1.342	8.580	7.003	1.352		1200
2.001		SW08	8.580	7.003	1.352	8.911	6.941	1.745	,	1200
1.003		SW03	8.911	6.408	2.203	8.990	6.360	2.330		1200
3.000		SW17	8.530	7.760	0.620	8.530	7.717	0.663		1200
3.001		SW16	8.530	7.717	0.663	8.675	7.654	0.871		1200
3.002		SW15	8.675	7.654	0.796	8.675	7.617	0.833		1200
3.003		SW14	8.675	7.617	0.758	8.640	7.580	0.760		1200
3.004		SW13	8.640	7.580	0.760	8.640	7.480	0.860		1200
3.005		SW12	8.640	7.480	0.860	9.300	7.390	1.610		1200
4.000		SW10	9.300	7.450	1.625	9.300	7.386	1.689		1800
3.006		SW11	9.300	7.390	1.610	8.900	7.299	1.301		1800
3.007		SW07	8.900	7.299	1.301	8.990	6.360	2.330		1200
1.004		SW02	8.990	6.360	2.030	9.000	6.229	2.171		1800
5.000		SW18	9.000	6.280	2.495	9.000	6.229	2.546		1800
1.005		SW01	9.000	6.229	2,171	9.000	5.990	2.410		1800
1.006	tubosider		9.000	5.400	1.200	9.000	5.350	1.250		10500
1,007		20	9.000	5.300	3,550	9.300	5.000	4,150	5	1500

BX03

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The Studio 51 Brookfield Road	BARRY EFW SURFACE DRAINAGE	TV BERO
Cheadle SK8 1ES		
Date 06/07/2017	Designed By HA	
File MODEL HA1.SUM	Checked By MS	
Micro Drainage	Simulation W.11.2	

MANHOLE SCHEDULES

M/Hole Number	Cover Level (m)	M/Hole Depth (m)	M/Hole Diam.,L*W (mm)	PN	Pipes Out IL.(m)	D (mm)	PN	Pipes In IL.(m)	D (mm)
SW06	8.350	1.690	1200	1.000	6,660	225			
SW05	8.440	1.884	1200	1.001	6.556	300	1,000	6.556	225
SW04	8.477	1.975	1200	1.002	6.502	300	1.001	6.502	300
SW09	8.667	1.567	1200	2.000	7,100	225			
SW08	8.580	1,577	1200	2.001	7.003	225	2.000	7.003	225
SW03	8.911	2.503	1200	1.003	6.408	300	1.002 2.001	6.408 6.941	300 225
SW17	8.530	0.770	1200	3.000	7.760	150			
SW16	8.530	0.813	1200	3.001	7,717	150	3.000	7.717	150
SW15	8.675	1.021	1200	3.002	7.654	225	3.001	7.654	150
SW14	8.675	1.058	1200	3.003	7.617	300	3.002	7.617	225
SW13	8.640	1.060	1200	3.004	7.580	300	3.003	7.580	300
SW12	8.640	1.160	1200	3.005	7.480	300	3.004	7.480	300
SW10	9.300	1.850	1800	4.000	7.450	225			
SW11	9.300	1.910	1800	3.006	7.390	300	3.005 4.000	7.390 7.386	300 225
SW07	8.900	1,601	1200	3.007	7.299	300	3.006	7.299	300
SW02	8.990	2.630	1800	1.004	6.360	600	1.003 3.007	6.360 6.360	300 300
SW18	9.000	2,720	1800	5.000	6.280	225			
SW01	9.000	2.771	1800	1.005	6.229	600	1.004 5.000	6.229 6.229	600 225
tubosider tank	9.000	3.600	10500	1.006	5.400	2400	1.005	5.990	600
20	9.000	3.700	1500	1.007	5.300	150	1.006	5.350	2400
Road	9.300	4.300	1200		OUTFALL		1.007	5.000	150

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	1.00		6.67		-0.290	0.000 0.000	0.51	0.0	186.6		
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	1.00	70	6.57	3	1.123	0.000	0.09	0.0	3.0	SURCH'ED	

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1.006	960 Winter	100	20%	1					
1.00	960 Winter	100	20%	1	1/15 Sur	nmer			
Lvl	Water Lv]	. Surcharg	ed Flood	ded	Flow/	Overflow	Pipe Flow	-	
Ex.	N (m)	Depth (m				(1/s)	(1/s)	Status	
	а		 c. 			 	14. L	X INT TO T	
			33 Ú				42 38 2	ala ing	
2.	000 7.2	96 -0.0	29 0.	.000	0.83	0.0	23.1	ОК	
2.	(1. 1.2 (1.7 − 5.3		11. A. Go 2		1.64 1.59	· · · · · ·	2.53	ALA ANACHTEC	
	000 7.9	64 0.0	54 0.	.000	0.34	0.0	3.2	SURCH'ED	
	001 7.9 002 7.9			.000	0.51 0.58	0.0		SURCH'ED SURCH'ED	
3.	003 7.9	25 0.0	08 0.	.000	0.48	0.0	24.2	SURCH'ED	
	004 7.9 005 .8			.000	0.78	0.0	45.4	SURCH'ED	
4.	000 7.7	95 0.1	.20 0.	.000	0.72	0.0	19.2	SURCH'ED	
3.	°.∦. 007 7.4			. 000 .000	0.36	0.0	87.1	ОК	
1.	004 6.8	56 -0.1	.04 0.	.000	0.07	0.0	26.3	ОK	
	000 6.8 005 6.8			.000	0.04 0.06	0.0 0.0	1.3 27.4	SURCH'ED SURCH'ED	
1.	006 6.8	56 -0.9	044 0	.000	0.00	0.0	21.6	ОК	
1.	007 6.8	56 1.4	106 0.	.000	0.09	0.0	3.0	SURCH'ED	
1									



REPORT

APPENDIX C – ACO DRAINAGE DESIGN OUTPUT

ACO Design

Hydraulic Calculation:

+ PROJECT DETAILS

Project Name: Barry EFW

Designer: Haroon Ashraf MS Date: 11-07-2017

Run No.: 3 Option No.: A

RAINFALL INTENSITY

RETURN PERIOD	30 years	
R	0.34	
M5-60	19.0	(mm/h)

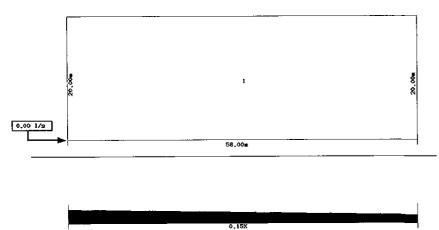
RAINFALL DATA

Duration	Intensity (mm/h)
a spilling a start	
10 mins	82.54
15 mins	71.62
30 mins	47.43
1 hour	29.91
2 hours	18.59
4 hours	11.08
6 hours	8.13
10 hours	5.57
24 hours	2.87
48 hours	1.63

RAINFALL SPECIFIED

ALLOWANCE FOR CLIMATE CHANGE	20.0	%
DESIGN RAINFALL INTENSITY	126.78	(mm/h)
	0.0352	l∕s m²

CHANNEL LAYOUT



ACO Technologies plc ACO Business Park Hitchin Road, Shefford Bedfordshire SG17 5TE

Tel: +44 (0)1462 816666 Fax: +44 (0)1462 815895



+ PROJECT DETAILS

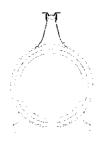
Project Name: Barry EFW

Designer:	Haroon Ashraf/M5
Date:	11-07-2017

+ INPUT

Channel System:

Qmax



	1	2	3	4	5	6	7	8	9	10
System	Qmax 150	Qmax 225	Qmax 350							
W – Width (mm)	150	225	350							
H – Invert (mm)	350	425	550							
Length (m)	8	34	16							

Kinematic Viscosity (m²/s)1.14x10^-6Rainfall Intensity (I/s x m)0.0352 (= 126.78mm/h)

Area Drained (m ²)	1160.00
Impermeability	1.00
Channel Length (m)	58.00

+ RESULTS

Outflow (I/s):	40.893
Max. Velocity (m/s):	1.06
Min. Freeboard (m):	0.000
Percentage Capacity (%):	99.97%
Max. Valid Length (m):	58.00 (Full Length)

Notes:

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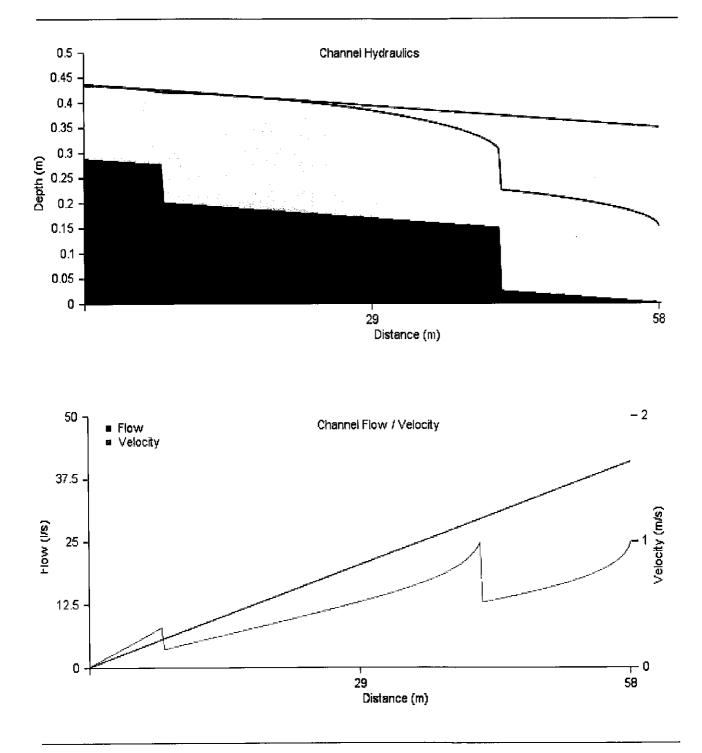




+ PROJECT DETAILS



Designer: Haroon Ashraf / ルク Date: 11-07-2017



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Tel: +44 (0)1462 816666 Fax: +44 (0)1462 815895

+ PROJECT DETAILS

Project Name:



CXOH

Haroon Ashraf $/\mu$? Designer: 11-07-2017

Barry EFW

Date:

DISCLAIMER

This simplified estimate of storage volume determines the largest volume required using the rainfall intensities for a range of different rainfall durations, for the location and return period specified by the designer. The type of flow control device is not known, so the calculation assumes a constant rate of outfall from the storage volume for the whole duration of the storm. Please contact ACO Design Services for further advice and details of the ACO Q-Brake Vortex Flow Control and the ACO StormBrixx Cellular Storage Tank.

INPUT		OUTPUT				
CATCHMENT AREA	(m²) I/s	R	1/s	STORAGE VO CURRENT CH		 m³
OUTFLOW	113	M5-60	mm/h	RETURN PER	IOD	
RESULT				CLIMATE CHA	NGE	%
		DURATION		NTENSITY nm/h	REQUIRED STORAGE VOLUME (m³)	

No Attenuation Calculation Performed.

ACO Design

Hydraulic Calculation:

+ PROJECT DETAILS

Project Name: Barry EFW

AC

Desig	ner: Matthew Spencer / HA
Date:	23-08-2016

Run No.: 4 Option No.: A

RAINFALL INTENSITY

RETURN PERIOD	30 years	
R	0.34	
M5-60	19.0	(mm/h)

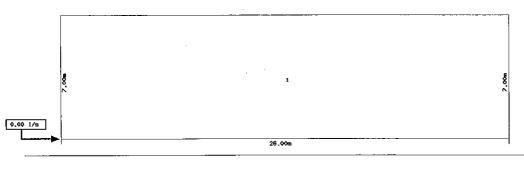
RAINFALL DATA

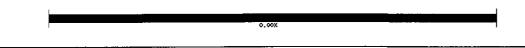
Duration	Intensity (mm/h)
	States and U. Albara
10 mins	82.54
15 mins	71.62
30 mins	47.43
1 hour	29.91
2 hours	18.59
4 hours	11.08
6 hours	8.13
10 hours	5.57
24 hours	2.87
48 hours	1.63

RAINFALL SPECIFIED

ALLOWANCE FOR CLIMATE CHANGE	20.0	%
DESIGN RAINFALL INTENSITY	126.78	(mm/h)
	0.0352	l∕s m²







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Tel: +44 (0)1462 816666 Fax: +44 (0)1462 815895

+ PROJECT DETAILS

Project Name: Barry EFW

Designer: Matthew Spencer / +1 A Date: 23-08-2016

+ INPUT

Channel System:

Qmax



	1	2	3	4	5	6	7	8	9	10
System	Qmax 150									
W – Width (mm)	150									
H – Invert (mm)	350									
Length (m)	26.00									

Rainfall Intensity (I/s x m) 0.0352 (= 126.78mm/h)

Area Drained (m²)	182.00	
Impermeability	1.00	
Channel Length (m)	26.00	

+ RESULTS

Outflow (I/s):	6.416
Max. Velocity (m/s):	0.69
Min. Freeboard (m):	0.022
Percentage Capacity (%):	90.70%
Max. Valid Length (m):	26.00 (Full Length)

Notes:

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Tel: +44 (0)1462 816666 Fax: +44 (0)1462 815895 email: technologies@aco.co.uk www.aco.co.uk

CXOC



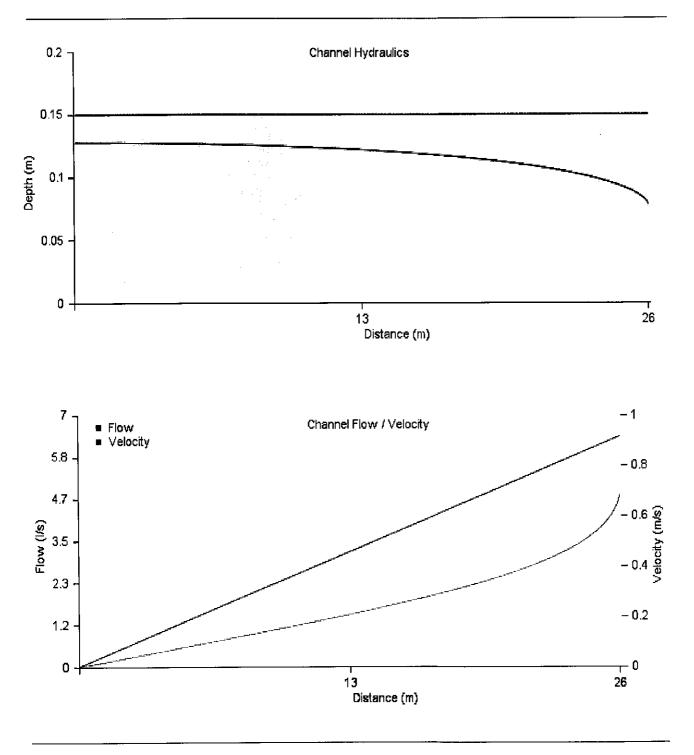
+ PROJECT DETAILS



Project Name:

Barry EFW

Designer:Matthew Spencer / HADate:23-08-2016



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Tel: +44 (0)1462 816666 Fax: +44 (0)1462 815895 email: technologies@aco.co.uk www.aco.co.uk

CXOT

+ PROJECT DETAILS





Designer:	Matthew Spencer / HA
Date:	23-08-2016

DISCLAIMER

This simplified estimate of storage volume determines the largest volume required using the rainfall intensities for a range of different rainfall durations, for the location and return period specified by the designer. The type of flow control device is not known, so the calculation assumes a constant rate of outfall from the storage volume for the whole duration of the storm. Please contact ACO Design Services for further advice and details of the ACO Q-Brake Vortex Flow Control and the ACO StormBrixx Cellular Storage Tank.

INPUT		OUTPUT				
CATCHMENT AREA	(m²)	R	l/s	STORAGE VO CURRENT CH		m³
MAX PERMITTED OUTFLOW	l/s	M5-60	mm/ł			
RESULT				CLIMATE CHA	NGE	%
		DURATION		INTENSITY mm/h	REQUIRED STORAGE VOLUME (m³)	

No Attenuation Calculation Performed.

ACO Design

Hydraulic Calculation:

+ PROJECT DETAILS

Project Name: Barry EFW

Run No.: 5 Option No.: A

RAINFALL INTENSITY

RETURN PERIOD	30 years	
R	0.34	
M5-60	19.0	(mm/h)

Designer:

Date:

WARNING - CRITICAL FLOW ENCOUNTERED

Matthew Spencer / Htk

23-08-2016

The flow in this channel varies between sub-critical and super-critical conditions so the depth / velocity calculation is not precise. However, your selected channel will have sufficient capacity to accommodate the defined flow. Note that optimised solutions will only suggest non-turbulent, sub-critical flow regimes.

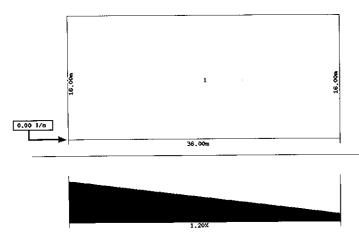
RAINFALL DATA

Duration	Intensity (mm/h)
Configuration	ana ang 101/11/14
10 mins	82.54
15 mins	71.62
30 mins	47.43
1 hour	29.91
2 hours	18.59
4 hours	11.08
6 hours	8.13
10 hours	5.57
24 hours	2.87
48 hours	1.63

RAINFALL SPECIFIED

ALLOWANCE FOR CLIMATE CHANGE	20.0	%
DESIGN RAINFALL INTENSITY	126.78	(mm/h)
	0.0352	l∕s m²

CHANNEL LAYOUT



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CX(O

ACO Design Hydraulic Calculation:

+ PROJECT DETAILS

Project Name: Barry EFW

Designer:Matthew Spencer / HADate:23-08-2016

+ INPUT

Channel System:



	1	2	3	4	5	6	7	8	• 9	10
System	Qmax 225									
W – Width (mm)	225									
H – Invert (mm)	425									
Length (m)	36.00									

Kinematic Viscosity (m²/s)1.14x10^-6Rainfall Intensity (I/s x m)0.0352 (= 126.78mm/h)

Qmax

Area Drained (m²)	576.00
Impermeability	1.00
Channel Length (m)	36.00

+ RESULTS

Outflow (I/s):	20.305
Max. Velocity (m/s):	0.91
Min. Freeboard (m):	0.102
Percentage Capacity (%):	55.98%
Max. Valid Length (m):	36.00 (Full Length)

Notes:

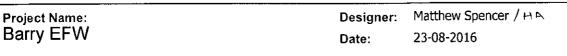
ACO Technologies plc ACO Business Park Hitchin Road, Shefford Bedfordshire SG17 5TE

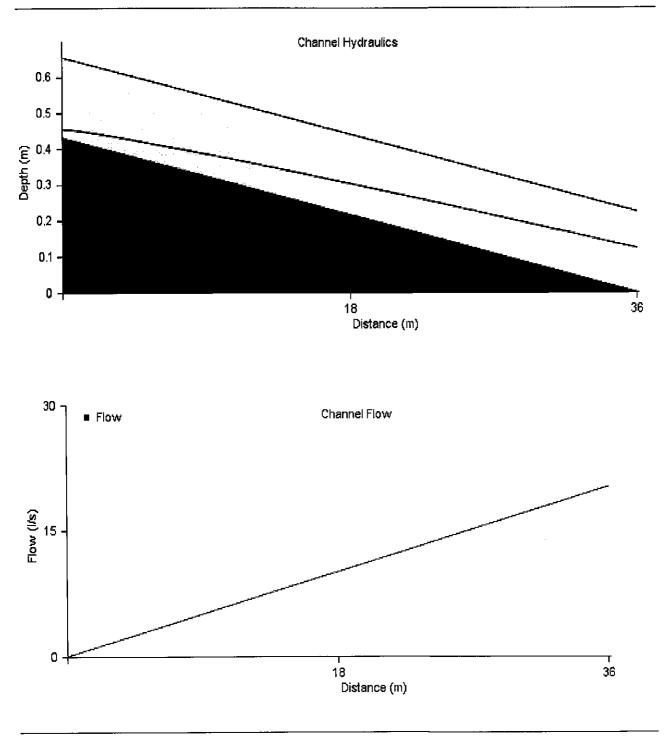
Tel: +44 (0)1462 816666 Fax: +44 (0)1462 815895





+ PROJECT DETAILS





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+ PROJECT DETAILS



Designer:	Matthew Spencer / μ^{R}
Date:	23-08-2016

Project Name: Barry EFW

DISCLAIMER

This simplified estimate of storage volume determines the largest volume required using the rainfall intensities for a range of different rainfall durations, for the location and return period specified by the designer. The type of flow control device is not known, so the calculation assumes a constant rate of outfall from the storage volume for the whole duration of the storm. Please contact ACO Design Services for further advice and details of the ACO Q-Brake Vortex Flow Control and the ACO StormBrixx Cellular Storage Tank.

INPUT		OUTPUT					
CATCHMENT AREA MAX PERMITTED OUTFLOW	(m²) I/s	R M5-60	l/s mm/	(STORAGE VO CURRENT CH RETURN PER	ANNEL	 m³
RESULT				(CLIMATE CHA	NGE	%
		DURATION		INTE mm/	ENSITY h	REQUIRED STORAGE VOLUME (m³)	

No Attenuation Calculation Performed.

ACO Design

Hydraulic Calculation:

CKIS

14

%

(mm/h)

l/s m²

20.0

126.78

0.0352

ACO

+ PROJECT DETAILS

Project Name: Barry EFW Designer: Haroon Ashraf Date: 11-07-2017

RAINFALL SPECIFIED

ALLOWANCE FOR

CLIMATE CHANGE

DESIGN RAINFALL

INTENSITY

Run No.: 6 Option No.: A

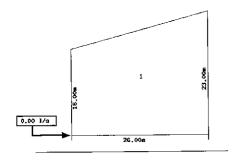
RAINFALL INTENSITY

RETURN PERIOD	30 years	
R	0.34	
M5-60	19.0	(mm/h)

RAINFALL DATA

Duration	Intensity (mm/h)
	an an children an
10 mins	82.54
15 mins	71.62
30 mins	47.43
1 hour	29.91
2 hours	18.59
4 hours	11.08
6 hours	8.13
10 hours	5.57
24 hours	2.87
48 hours	1.63

CHANNEL LAYOUT





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CX(4

ACO Design Hydraulic Calculation:

+ PROJECT DETAILS

Project Name: Barry EFW

Designer: Haroon Ashraf / ກ 5 Date: 11-07-2017

+ INPUT

Channel System:



	1	2	3	4	5	6	7	8	9	10
System	Qmax 150	Qmax 225								
W – Width (mm)	150	225								
H – Invert (mm)	350	425								
Length (m)	20	6								

Kinematic Viscosity (m^2/s)1.14x10^-6Rainfall Intensity (I/s x m)0.0352 (= 126.78mm/h)

Qmax

Area Drained (m²)	507.00	
Impermeability	1.00	
Channel Length (m)	26.00	

+ RESULTS

Outflow (I/s):	17.876
Max. Velocity (m/s):	0.94
Min. Freeboard (m):	0.010
Percentage Capacity (%):	96.99%
Max. Valid Length (m):	26.00 (Full Length)

Notes:

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Tel: +44 (0)1462 816666 Fax: +44 (0)1462 815895

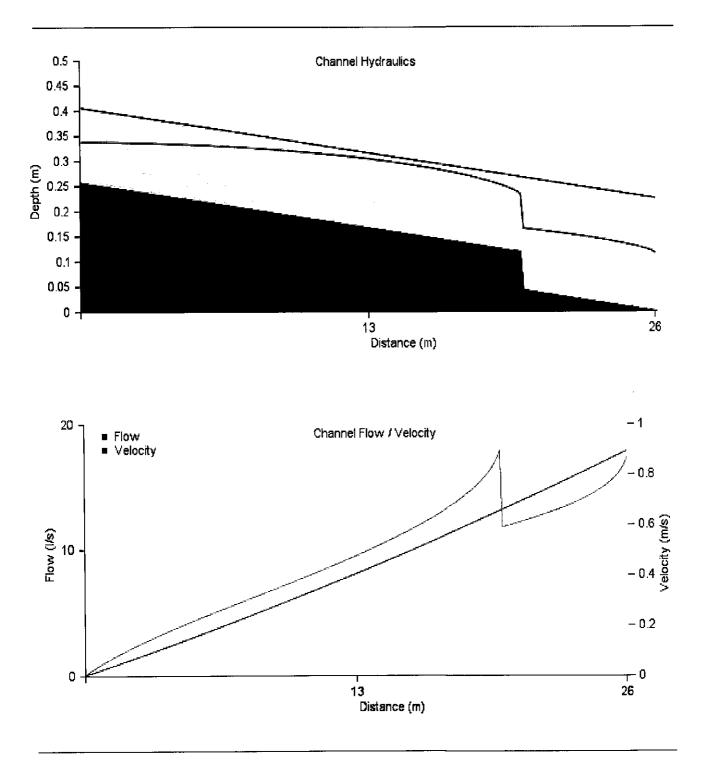




+ PROJECT DETAILS



Haroon Ashraf $/M^{2}$ Designer: 11-07-2017 Date:



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Tel: +44 (0)1462 816666 Fax: +44 (0)1462 815895

+ PROJECT DETAILS



ACC

Designer:	Haroon Ashraf $/M_{2}$
Date:	11-07-2017

DISCLAIMER

This simplified estimate of storage volume determines the largest volume required using the rainfall intensities for a range of different rainfall durations, for the location and return period specified by the designer. The type of flow control device is not known, so the calculation assumes a constant rate of outfall from the storage volume for the whole duration of the storm. Please contact ACO Design Services for further advice and details of the ACO Q-Brake Vortex Flow Control and the ACO StormBrixx Cellular Storage Tank.

INPUT		OUTPUT					
CATCHMENT AREA	(m²)	R	l/s		AGE VO	LUME OF ANNEL	m³
MAX PERMITTED OUTFLOW	l/s	M5-60	mm/ł	n RETU	RN PERI	OD	
RESULT				CLIMA	TE CHA	NGE	%
		DURATION		INTENSIT mm/h	Y	REQUIRED STORAGE VOLUME (m³)	

No Attenuation Calculation Performed.

ACO Design

Hydraulic Calculation:

+ PROJECT DETAILS

Project Name: Barry EFW

Run No.: 7 Option No.: A

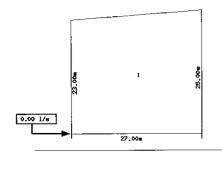
RAINFALL INTENSITY

RETURN PERIOD	30 years	
R	0.34	
M5-60	19.0	(mm/h)

RAINFALL DATA

Duration	Intensity (mm/h)
	And States and All Market
10 mins	82.54
15 mins	71.62
30 mins	47.43
1 hour	29.91
2 hours	18.59
4 hours	11.08
6 hours	8.13
10 hours	5.57
24 hours	2.87
48 hours	1.63

CHANNEL LAYOUT





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Tel: +44 (0)1462 816666 Fax: +44 (0)1462 815895 email: technologies@aco.co.uk www.aco.co.uk



Designer: Matthew Spencer / HA Date: 23-08-2016

RAINFALL SPECIFIED

ALLOWANCE FO	Z	20.0	%
DESIGN RAINFA INTENSITY	ALL 1:	26.78	(mm/h)
	0.	.0352	l/s m²

+ PROJECT DETAILS

Project Name: Barry EFW

+ INPUT

Channel System:

Qmax



	1	2	3	4	5	6	7	8	9	10
System	Qmax 225									
W – Width (mm)	225									
H – Invert (mm)	425									
Length (m)	27.00									

Rainfall Intensity (I/s x m) 0.0352 (= 126.78mm/h)

Area Drained (m²)	648.00	
Impermeability	1.00	
Channel Length (m)	27.00	

+ RESULTS

Outflow (I/s):	0.000
Max. Velocity (m/s):	0.00
Min. Freeboard (m):	0.000
Percentage Capacity (%):	0.00%
Max. Valid Length (m):	27.00 (Full Length)

Notes:

ACO Technologies plc ACO Business Park Hitchin Road, Shefford Bedfordshire SG17 5TE

Tel: +44 (0)1462 816666 Fax: +44 (0)1462 815895

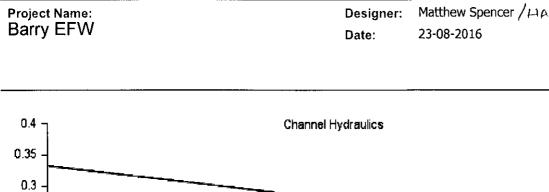


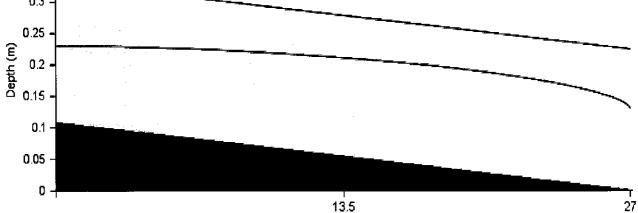
ACO Design

Hydraulic Calculation:

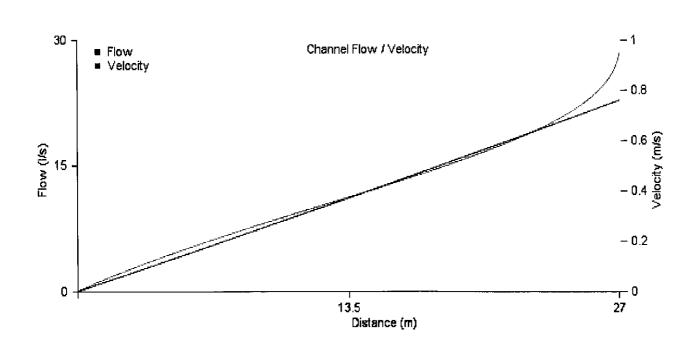


+ PROJECT DETAILS





13.5 Distance (m)



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CX19

+ PROJECT DETAILS



Project Name:Designer:Matthew Spencer / HABarry EFWDate:23-08-2016

DISCLAIMER

This simplified estimate of storage volume determines the largest volume required using the rainfall intensities for a range of different rainfall durations, for the location and return period specified by the designer. The type of flow control device is not known, so the calculation assumes a constant rate of outfall from the storage volume for the whole duration of the storm. Please contact ACO Design Services for further advice and details of the ACO Q-Brake Vortex Flow Control and the ACO StormBrixx Cellular Storage Tank.

INPUT		OUTPUT				
CATCHMENT AREA	(m²)	R	l/s	STORAGE VO CURRENT CH		m³
MAX PERMITTED OUTFLOW	l/s	M5-60	mm/h	RETURN PER	IOD	
RESULT				CLIMATE CHA	NGE	%
		DURATION		ITENSITY m/h	REQUIRED STORAGE VOLUME (m³)	

No Attenuation Calculation Performed.

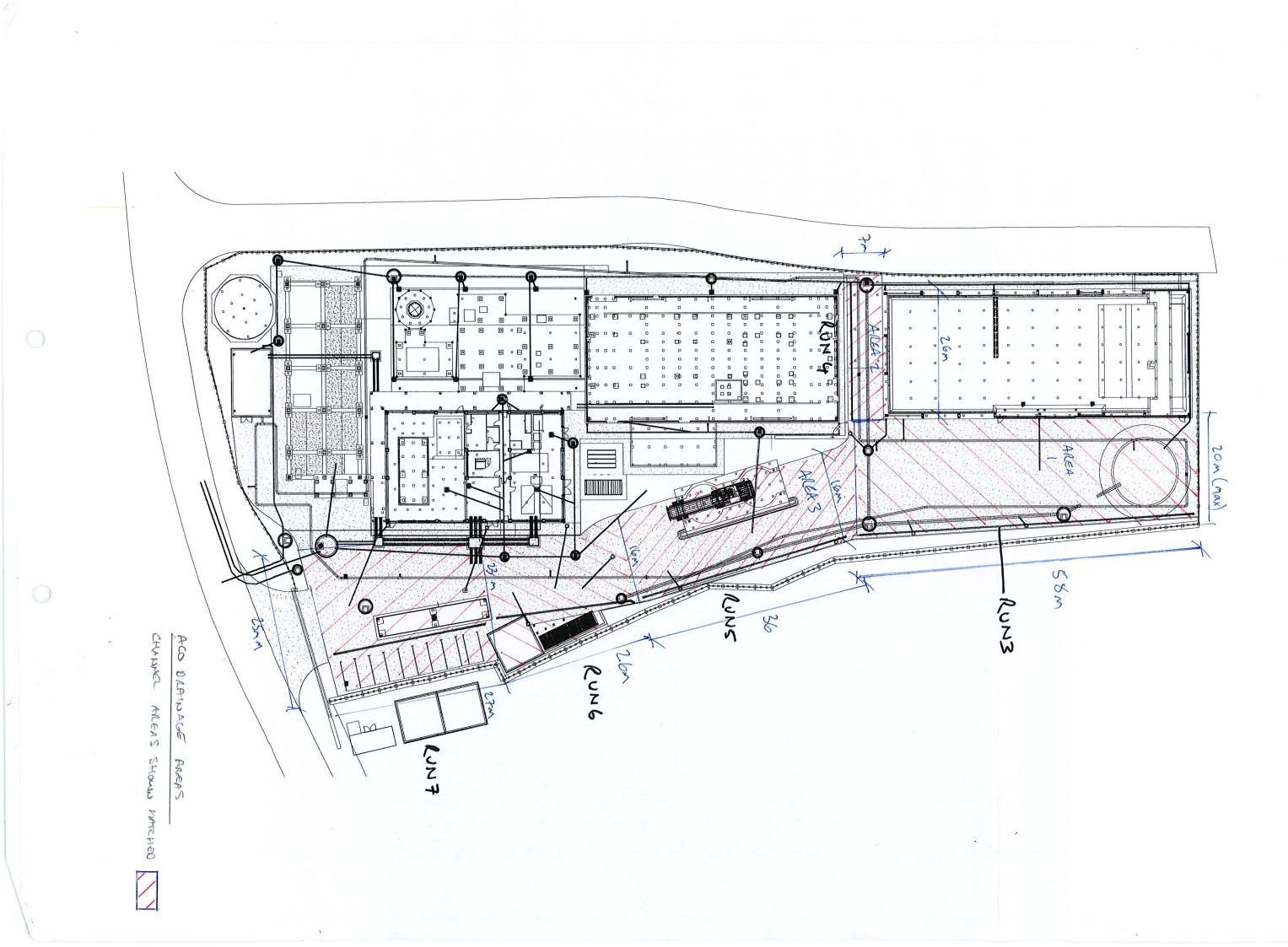
Tel: +44 (0)1462 816666 Fax: +44 (0)1462 815895 email: technologies@aco.co.uk www.aco.co.uk

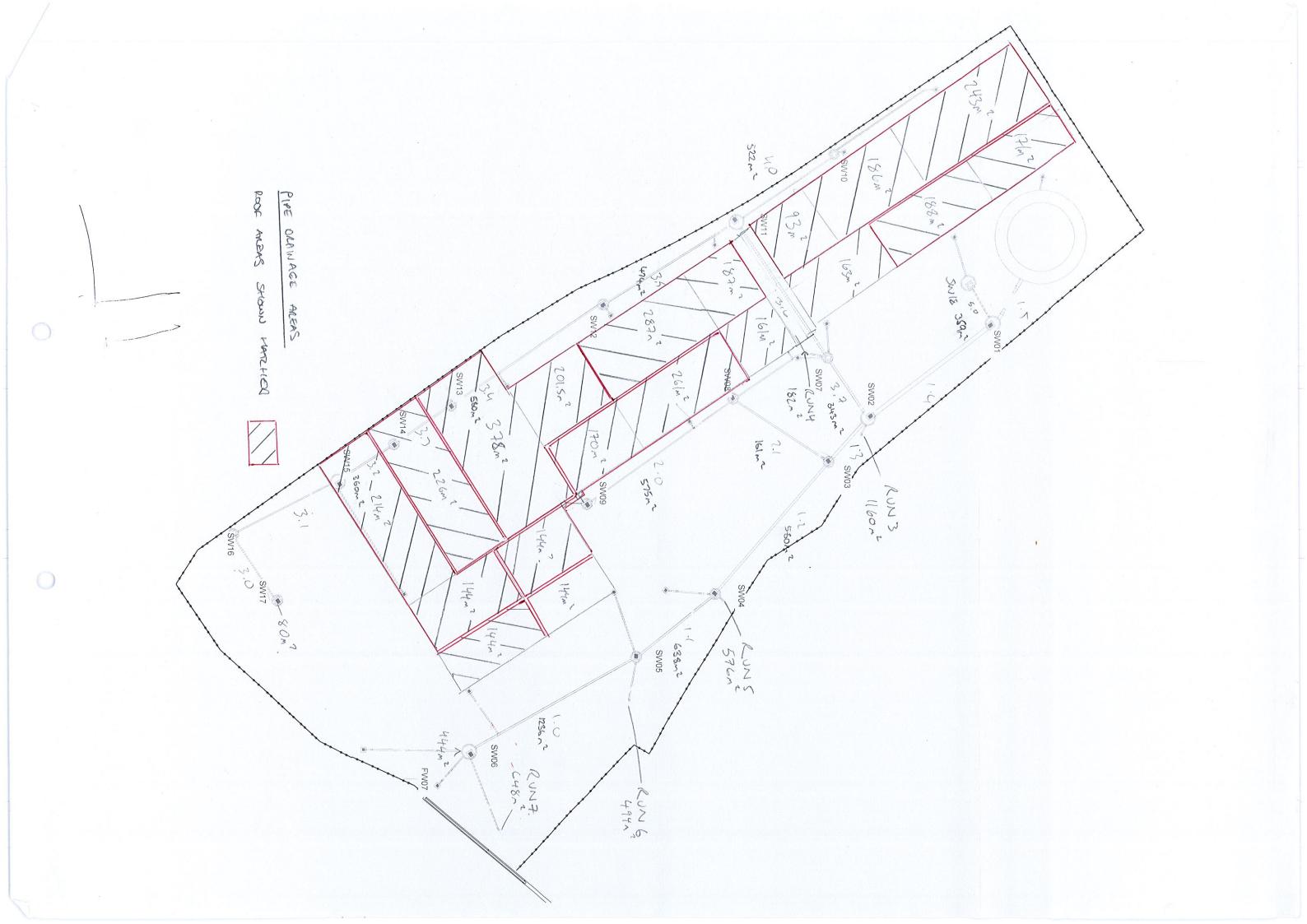


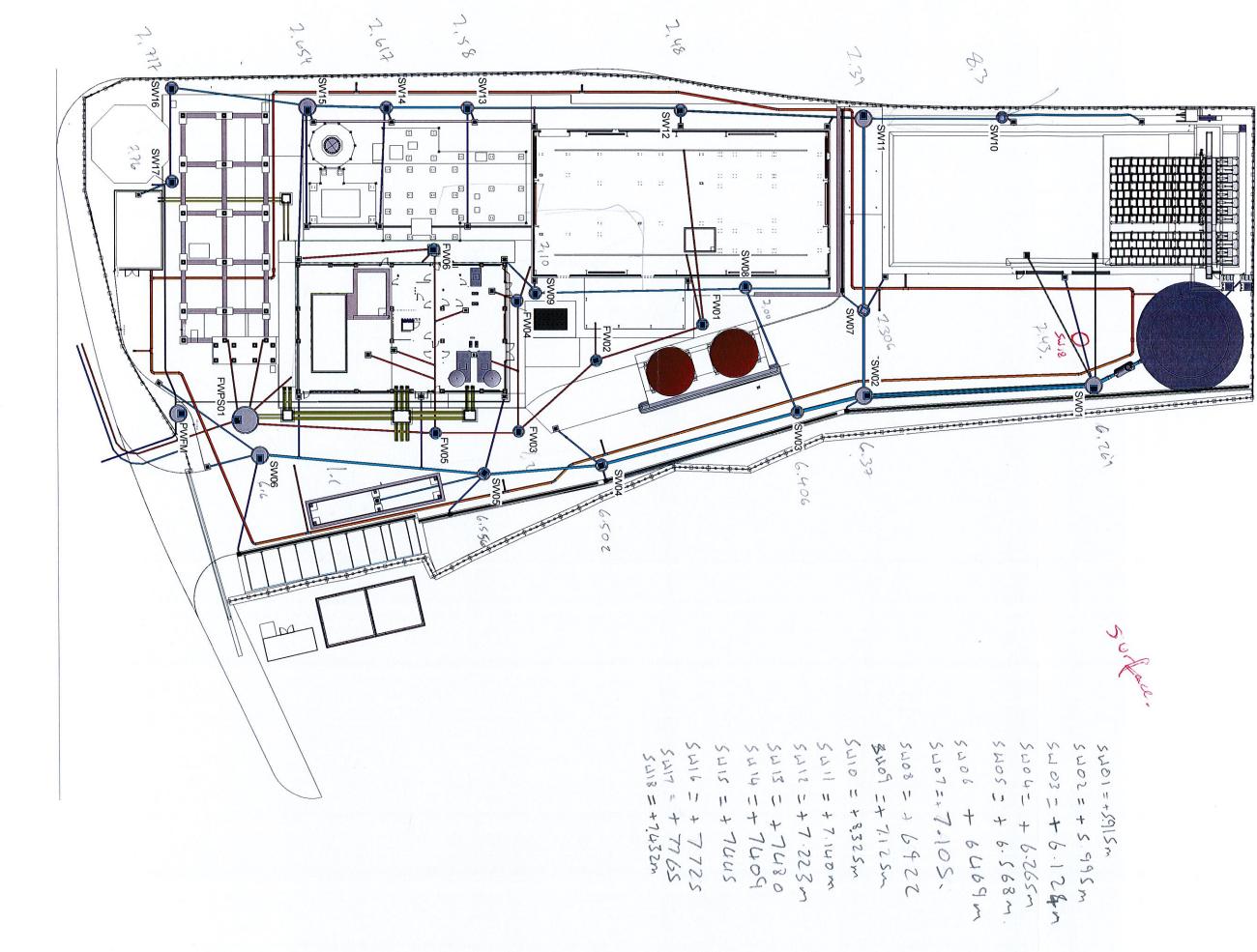
BARRY_01_REP_01_20063

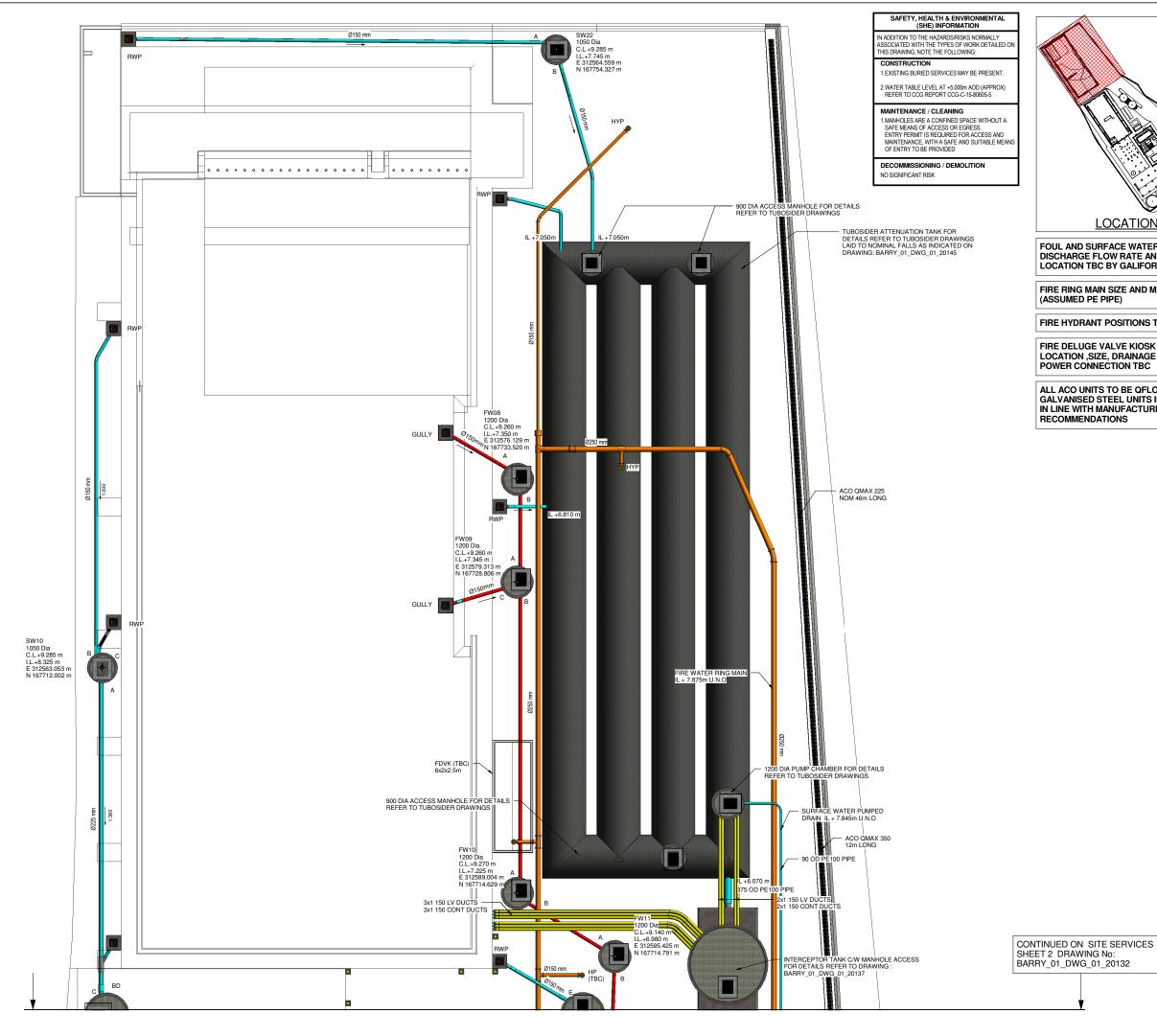
REPORT

APPENDIX D – REFERENCE DRAWINGS











DO NOT SCALE - IF IN DOUBT ASK NOTES

GENERAL

- 1. DO NOT SCALE FROM THIS DRAWING.
- 2. ALL DIMENSIONS ARE IN MILLIMETRES (mm) & ALL LEVELS ARE IN METRES (m) AOD, UNLESS STATED OTHERWISE.
- 3. ALL DIMENSIONS & LEVELS TO BE CHECKED ON SITE AND ANY DISCREPANCIES SHOULD BE REPORTED TO GHD LIVIGUNN.

 THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL RELEVANT PROJECT STANDARDS AND SPECIFICATIONS. DRAINAGE:

- ALL DRAINAGE WORKS TO BE CARRIED OUT IN ACCORDANCE WITH CESWI 7TH EDITION
- NEW DRAINS ARE UNDERGROUND PVC-U IIACCORDANCE WITH BS 4660. PROCESS WATER DRAINS TO BE HATHERNWARE CHEMICAL RESISTANCE TBC
- PIPE JOINTS IMMEDIATELY ADJACENT TO STRUCTURES AND CHAMBERS ARE FULLY ARTICULATED ROCKER PIPE JOINTS.
- ROCKER PIPES ARE 600mm LONG U.N.O ROCKER PIPES TO BE POSITIONED MINIMUM ACHIEVABLE DISTANCE FROM MANHOLE OR CHAMBER WITH ALLOWANCE FOR INSTALLATION OF FLEXIBLE JOINT
- ALL BURIED PIPES AND DUCTS UNDER STRUCTURE TO BE ENCASED IN MIN 150mm THICK C16/20 MASS CONCRETE. ALSO WHERE GROUND COVER IS LESS THAN 900mm
- DOUBLE STEPS SHALL BE PLASTIC ENCAPSULATED CARBON STEEL MANHOLE STEPS TO BS 1247-2. STEP IRONS AT 250/300 CENTRES HORIZONTALLY AND VERTICALLY MAY BE USED AS AN ALTERNATIVE TO DOUBLE STEPS STEPS
- PIPE BEDDING DETAILS SHOWN ON DRAWING: BARRY_01_DWG_01_20135

LEGEND:

- HYDRANT RING MAIN
 - SURFACE WATER DRAIN
- FOUL WATER DRAIN
- PROCESS WATER DRAIN (AS HATHERNWARE THERMACHEM PIPEWORK) ELECTRICAL DUCTS
- POTABLE WATER MAIN (AS PURITON BARRIER PIPE OSA)
- SURFACE WATER MANHOLE

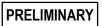
- SW SURFACE WATER MANHOLE FW FOUL WATER MANHOLE DP ELECTRICAL DRAWPIT RWP RAIN WATER PIPE GU ROAD GULLY HYP HYDRANT POINT (PROPOSEI IV ISOLATION VALVE (PROPOSI BD BACK DROP FDVK FIRE DELUGE VALVE KIOSK SURFACE WATER MANHOLE FOUL WATER MANHOLE ELECTRICAL DRAWPIT RAIN WATER PIPE ROAD GULLY HYDRANT POINT (PROPOSED) ISOLATION VALVE (PROPOSED) BACK DROD

REFERENCE DRAWINGS :

BARRY_01_DWG_01_20131 - SITE SERVICES GA SHEET 1 OF 4
BARRY_01_DWG_01_20132 - SITE SERVICES GA SHEET 2 OF 4
BARRY_01_DWG_01_20133 - SITE SERVICES GA SHEET 3 OF 4
BARRY_01_DWG_01_20134 - SITE SERVICES GA SHEET 4 OF 4
BARRY_01_DWG_01_20135 - SITE SERVICES DETAILS SHEET 1
BARRY_01_DWG_01_20136 - SITE SERVICES SCHEDULE

2D EXPORT FROM A 3D MODEL

DRAWING TO BE REPRODUCED IN COLOUR



SERVICES UPDATED UPDATED FOR ATTENUATION FIRST ISSUE Description	JW JW JW By	MS MS MS Chk	GB GB GB App
UPDATED FOR ATTENUATION	JW	MS	GB
SERVICES UPDATED	JW	MS	GB
FOUL DRAIN & SURFACE RISING MAIN UPDATED	JW	MS	GB
SERVICES & DNOC AREA UPDATED	JW	MS	GB



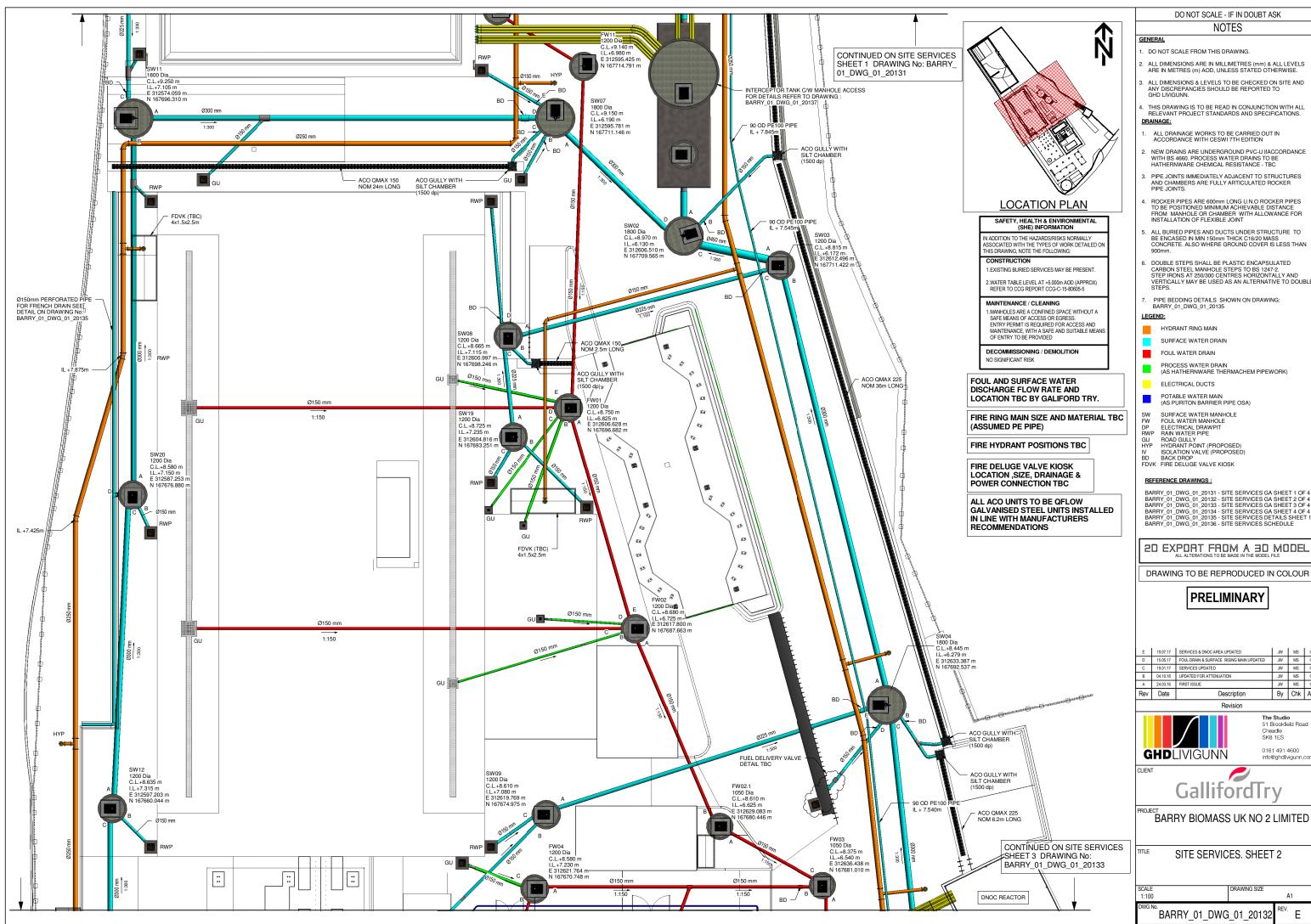
PROJECT BARRY BIOMASS UK NO 2 LIMITED

SITE SERVICES. SHEET 1

TITLE

SCALE DRAWING SIZE Δ1 1:100

REV. BARRY 01 DWG 01 2013 Е



DO NOT SCALE - IF IN DOUBT ASK

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- NEW DRAINS ARE LINDERGROUND PVC-IL IMCCORDANCE WITH BS 4660. PROCESS WATER DRAINS TO BE HATHERNWARE CHEMICAL RESISTANCE - TBC
- 8. PIPE JOINTS IMMEDIATELY ADJACENT TO STRUCTURES AND CHAMBERS ARE FULLY ARTICULATED ROCKER PIPE JOINTS.
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- PIPE BEDDING DETAILS SHOWN ON DRAWING: BARRY_01_DWG_01_20135

BARRY_01_DWG_01_20131 - SITE SERVICES GA SHEET 1 OF 4 BARRY_01_DWG_01_20132 - SITE SERVICES GA SHEET 2 OF 4 BARRY_01_DWG_01_20133 - SITE SERVICES GA SHEET 3 OF 4 BARRY_01_DWG_01_20134 - SITE SERVICES GA SHEET 3 OF 4 BARRY_01_DWG_01_20134 - SITE SERVICES GA SHEET 4 OF 4 BARRY_01_DWG_01_20136 - SITE SERVICES DETAILS SHEET 1 DWG_01_20136 SITE SERVICES DETAILS SHEET 1 BARRY_01_DWG_01_20136 - SITE SERVICES SCHEDULE

2D EXPORT FROM A 3D MODEL

DRAWING TO BE REPRODUCED IN COLOUR

PRELIMINARY

		Bavisian			
Rev	Date	Description	By	Chk	App
Α	24.03.16	FIRST ISSUE	JW	MS	GB
В	04.10.16	UPDATED FOR ATTENUATION	JW	MS	GB
С	19.01.17	SERVICES UPDATED	JW	MS	GB
D	15.05.17	FOUL DRAIN & SURFACE RISING MAIN UPDATED	JW	MS	GB
Е	19.07.17	SERVICES & DNOC AREA UPDATED	JW	MS	GB

The Studio 51 Brookfield Road Cheadle SK8 1ES

0161 491 4600 info@ghdlivigunn.cor

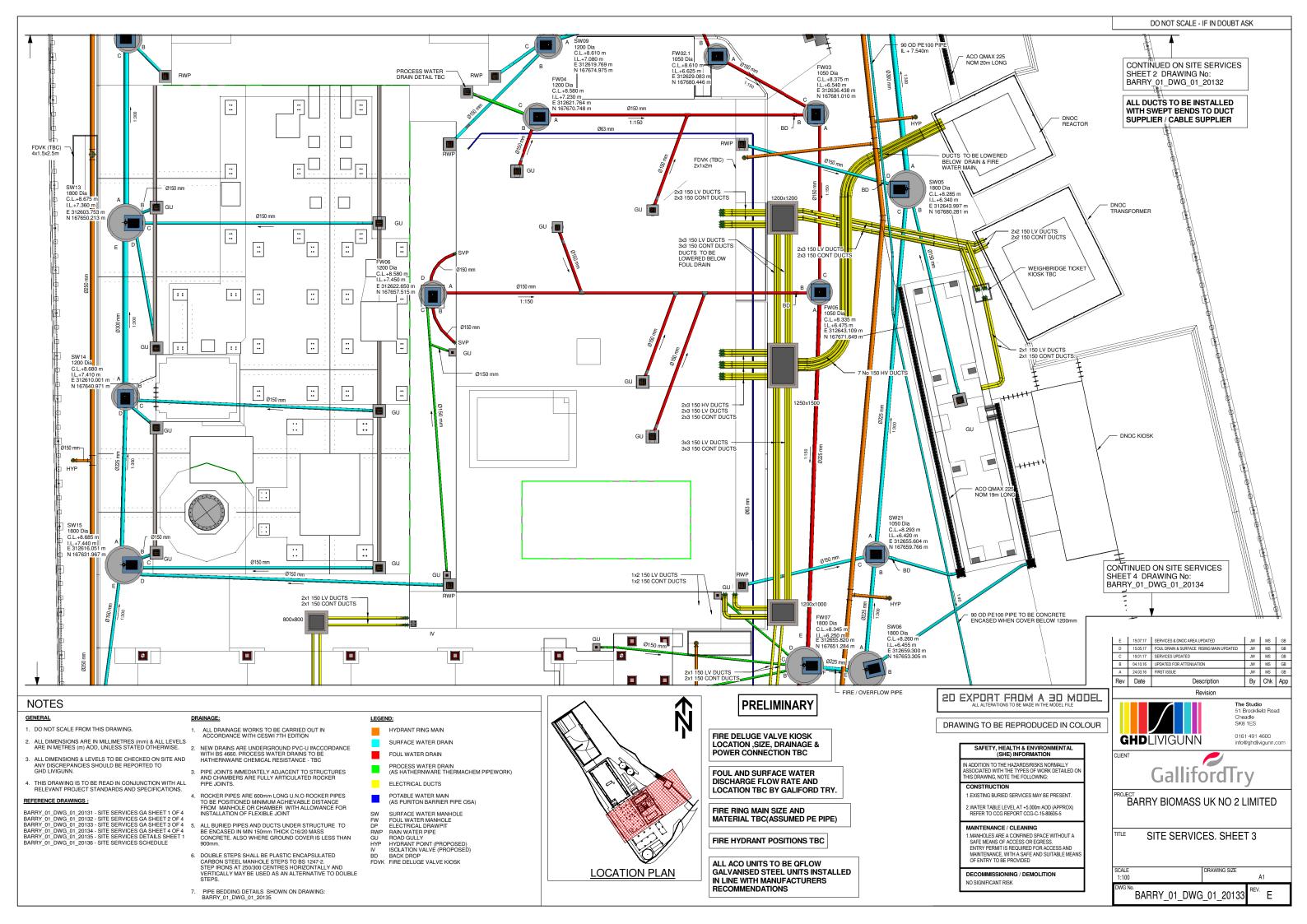
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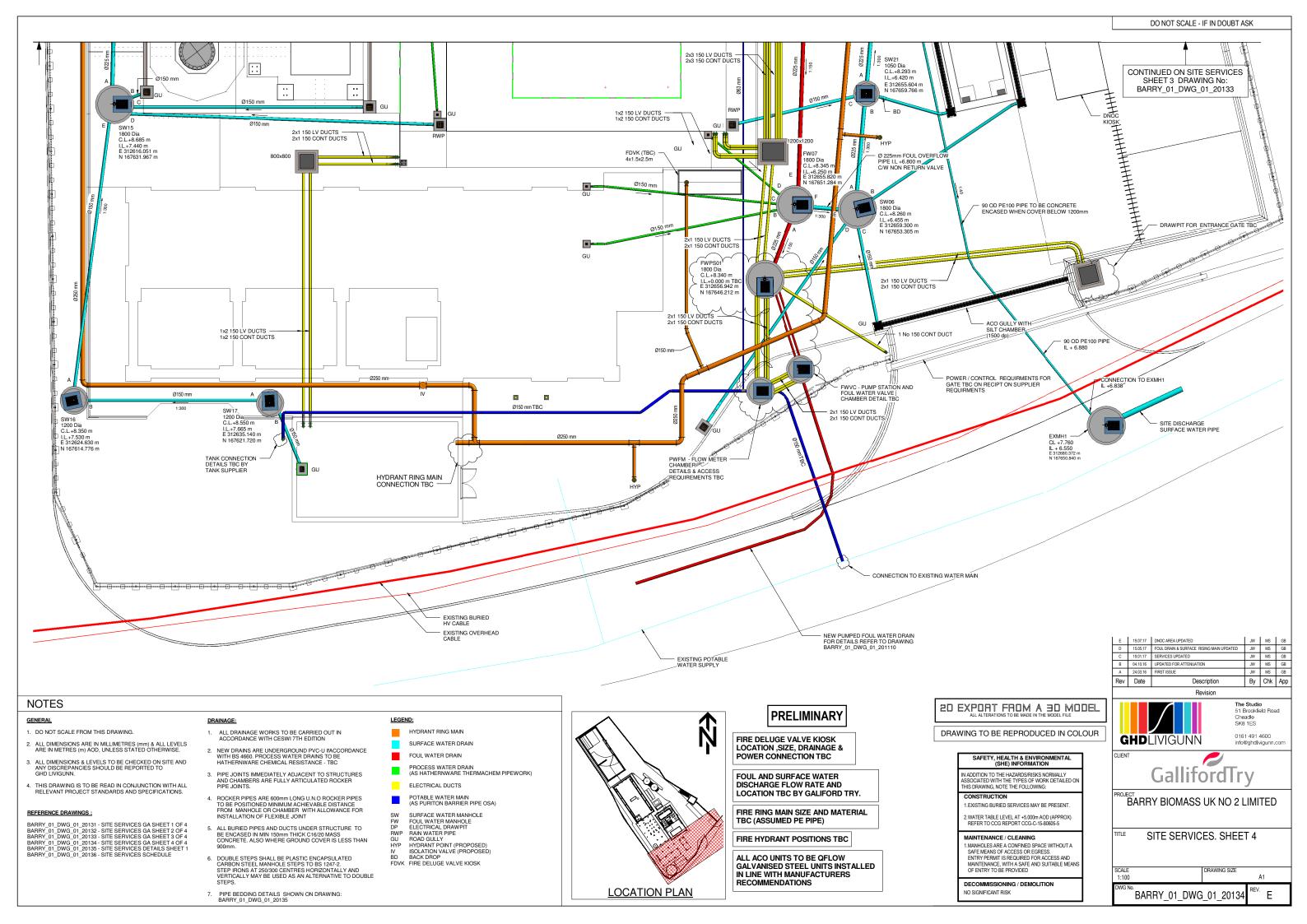
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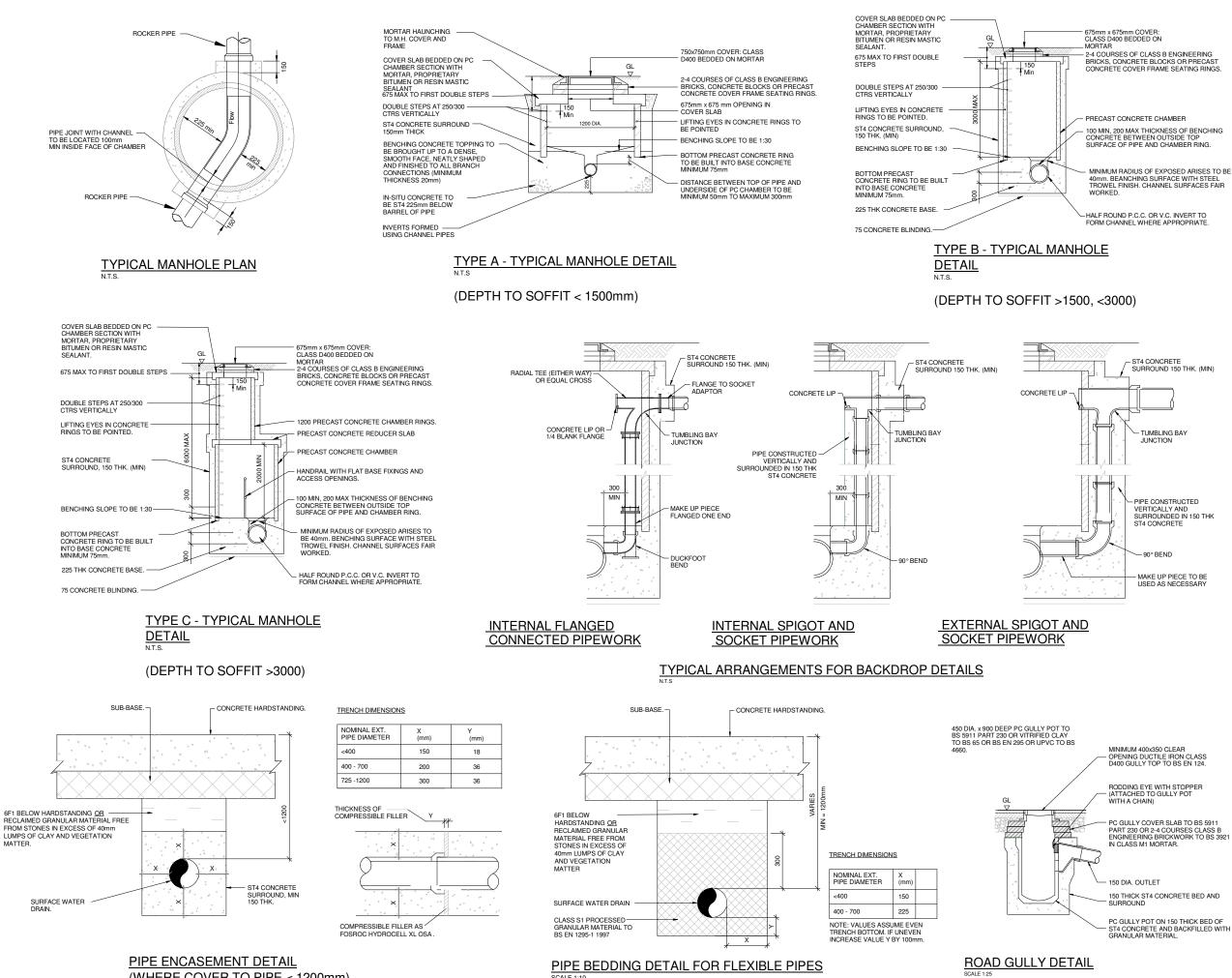
SITE SERVICES. SHEET 2

RAWING SIZE

REV. BARRY 01 DWG 01 20132







(WHERE COVER TO PIPE < 1200mm)

SCALE 1:10

PC GULLY COVER SLAB TO BS 5911 PART 230 OR 2-4 COURSES CLASS B ENGINEERING BRICKWORK TO BS 3921

150 THICK ST4 CONCRETE BED AND SURROUND

DO NOT SCALE - IF IN DOUBT ASK NOTES

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DRAINAG

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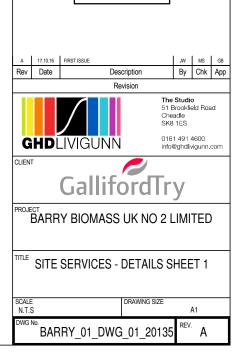
REFERENCE DRAWINGS :

BARRY_01_DWG_01_20131 - SITE SERVICES GA SHEET 1 OF 4 BARRY_01_DWG_01_20132 - SITE SERVICES GA SHEET 2 OF 4 BARRY_01_DWG_01_20133 - SITE SERVICES GA SHEET 3 OF 4 BARRY_01_DWG_01_20134 - SITE SERVICES GA SHEET 4 OF 4 BARRY_01_DWG_01_20135 - SITE SERVICES GA SHEET 4 OF 4 BARRY_01_DWG_01_20136 - SITE SERVICES DETAILS SHEET 2

SEE DETAILED DRAWINGS FOR HATHERNWARE THERMACHEM GULLY DETAILS AND STD GULLY DETAILS WHERE CAST INTO SLABS.

2D EXPORT FROM A 3D MODEL

PRELIMINARY



ANHOLE No.	COVER LEVEL (m)	DIAGRAMATIC LAYOUT	EASTING (m)	NORTHING (m)	INTERNAL DIAMETER (mm)	PIPE REF	PIPE INVERL Lvl (m)	BACKDROP ENTRY	PIPE DIAMETER (mm)	CHAMBER TYPE	COVER GRADE	MANHOLE No.	COVER LEVEL (m)	DIAGRAMATIC LAYOUT	EASTING (m)	NORTHING (m)	INTERNAL DIAMETER (mm)	PIPE REF	PIPE INVERL Lvl (m)	BACKDROP ENTRY	PIPE DIAMETER (mm)	CHAMBER TYPE	COV GRA
EXMH01	7.760	BOA	312680.372	167650.840	1800	A B	+6.550 +6.838	N/A N/A	90 300	EXISTING	D400	SW03	8.815	Â	312612.496	167711.422	1200	A B C	+6.172 +6.190 +6.580	N/A N/A N/A	450 300 225	TYPE B	D40
FW01	8.750		312606.628	167696.682	1200	A B C D E	+6.825 +7.000 +6.950 +6.840 +6.990	N/A N/A N/A N/A N/A	150 150 150 150 150 150	TYPE B	D400	SW04	8.445		312633.387	167692.537	1800	A B C D E	+6.279 +6.420 +6.293 +6.420 +7.260	N/A Y N/A Y Y	300 150 300 150 225	TYPE B	D40
FW02	8.680		312617.800	167687.663	1200	A B C D E	+6.725 +6.935 +6.910 +6.935 +6.735	N/A N/A N/A N/A	150 150 150 150 150	TYPE B	C250	SW05	8.285		312643.997	167680.281	1800	A B C D	+6.340 +7.210 +6.375 +6.358	N/A N/A N/A Y	300 150 225 150	TYPE B	D40
FW02.1	8.610	BOA .	312629.083	167680.446	1050	A B	+6.625 +6.643	N/A N/A	150 150	TYPE B	D400	SW06	8.260		312659.300	167653.305	1800	A B C D E	+6.455 +7.160 +7.155 +6.780 +6.750	N/A N/A N/A N/A	225 150 150 150 225	TYPE B	D4
FW03	8.375		312636.438	167681.010	1050	A B C	+6.540 +7.125 +6.582	N/A Y N/A	150 150 150	TYPE B	D400	SW07	9.150	D B A	312595.781	167711.146	1800	A B C D E	+6.190 +7.110 +7.100 +7.025 +7.100	N/A N/A Y Y	300 150 150 300 150	TYPE C	D
FW04	8.580	° O A	312621.764	167670.748	1200	A B C	+7.230 +7.290 +7.290	N/A N/A N/A	150 150 150	TYPE B	D400	SW08	8.665		312600.997	167698.246	1200	A B C D	+7.115 +7.165 +7.235 +7.455	N/A N/A N/A Y	225 150 225 150	TYPE B	D
W05	8.335	-BOA	312643.109	167671.649	1050	A B C	+6.475 +7.130 +6.435	N/A Y N/A	150 150 225	TYPE B	D400	SW09	8.610	C D A	312619.769	167674.975	1200	A B C	+7.080 +7.465 +7.645	N/A N/A N/A	225 150 150	TYPE B	C
W06	8.580		312622.650	167657.515	1200	A B C D	+7.450 +7.500 +7.500 +7.525	N/A N/A N/A N/A	150 150 150 150	TYPE A	D400	SW10	9.285	BK	312563.053	167712.002	1050	A B C	+8.325 +8.330 +8.330	N/A N/A N/A	225 150 150	TYPE A	
W07	8.345		312655.820	167651.284	1800	A B C D E F	+6.250 +7.300 +.7300 +6.550 +6.325 +6.800	N/A N/A N/A N/A N/A	225 150 150 225 225	TYPE B	D400	SW11	9.250		312574.059	167696.310	1800	A B C	+7.105 +7.120 +.7120	N/A N/A Y	300 300 225	TYPE B	
W08	9.260	A B	312576.129	167733.520	1200	A B	+7.355 +7.350	N/A N/A	150 150	TYPE A	D400	SW12	8.635		312597.203	167660.044	1200	A B C	+7.315 +7.325 +7.330	N/A N/A N/A	300 150 300	TYPE A	l
W09	9.260	C B	312579.317	167728.801	1200	A B C	+7.350 +7.345 +7.345	N/A N/A N/A	150 150 150	TYPE A	D400	SW13	8.675	Ĉ.	312603.753	167650.213	1800	A B C D E	+7.360 +7.440 +7.440 +7.450 +7.380	N/A N/A N/A N/A	300 150 150 150 300	TYPE A	l
W10	9.270	Â,	312589.004	167714.629	1200	A B	+7.230 +7.228	N/A N/A	150 150	TYPE A	D400	SW14	8.680		312610.001	167640.971	1200	A B C D	+7.410 +7.490 +7.490 +7.420	N/A N/A N/A N/A	300 150 150 225	TYPE A	
W11	9.140		312595.425	167714.791	1200	A B	+6.985 +6.980	N/A N/A	150 150	TYPE B	D400	SW15	8.685		312616.051	167631.967	1800	A B C D E	+7.440 +7.500 +7.500 +7.500 +7.460	N/A N/A N/A N/A	225 150 150 150 150	TYPE A	
'PS01	8.340	G A	312656.942	167646.212	1800	A B	TBC +6.300	N/A N/A	80 150	твс	D400	SW16	8.350	А. В.	312624.630	167614.776	1200	A B	+7.530 +7.625	N/A N/A	150 150	TYPE A	
wvc	0.000	B	312662.098	167642.735	1200	A B	TBC TBC	N/A N/A	150 150 100 150	TBC	D400	SW17	8.550	- Q _B	312635.140	167621.720	1200	A B	+7.665 +7.675	N/A N/A	150 150	TYPE A	
WFM	8.305		312660.730	167640.126	1200	A B	TBC TBC	N/A N/A	150 150	твс	D400	SW19	8.725	Â	312604.816	167693.251	1200	A B C	+7.235 +7.265 +7.275	N/A N/A N/A	225 150 150	TYPE A	
W02	8.970	D C C	312606.510	167709.565	1800	A B C D	+6.130 +6.160 +6.135 +6.135	N/A Y N/A N/A	375 150 450 300	TYPE B	D400	SW20	8.580		312587.253	167676.880	1200	A B C D	+7.150 +7.250 +7240 +7.250	N/A N/A N/A	300 150 300 150	TYPE B	[
I			1	1				1]	SW21	8.293		312655.604	167659.766	1050	A B C D	+6.420 +7.135 +6.430 +6.450	N/A Y N/A N/A	225 150 225 150	TYPE B	
												SW22	9.285	<u> </u>	312564.559	167754.327	1050	A B	+7.775 +7.745	N/A N/A	150 150	TYPE A	

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NOTES

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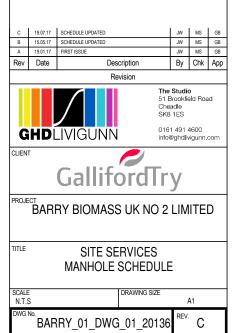
DRAINAGE:

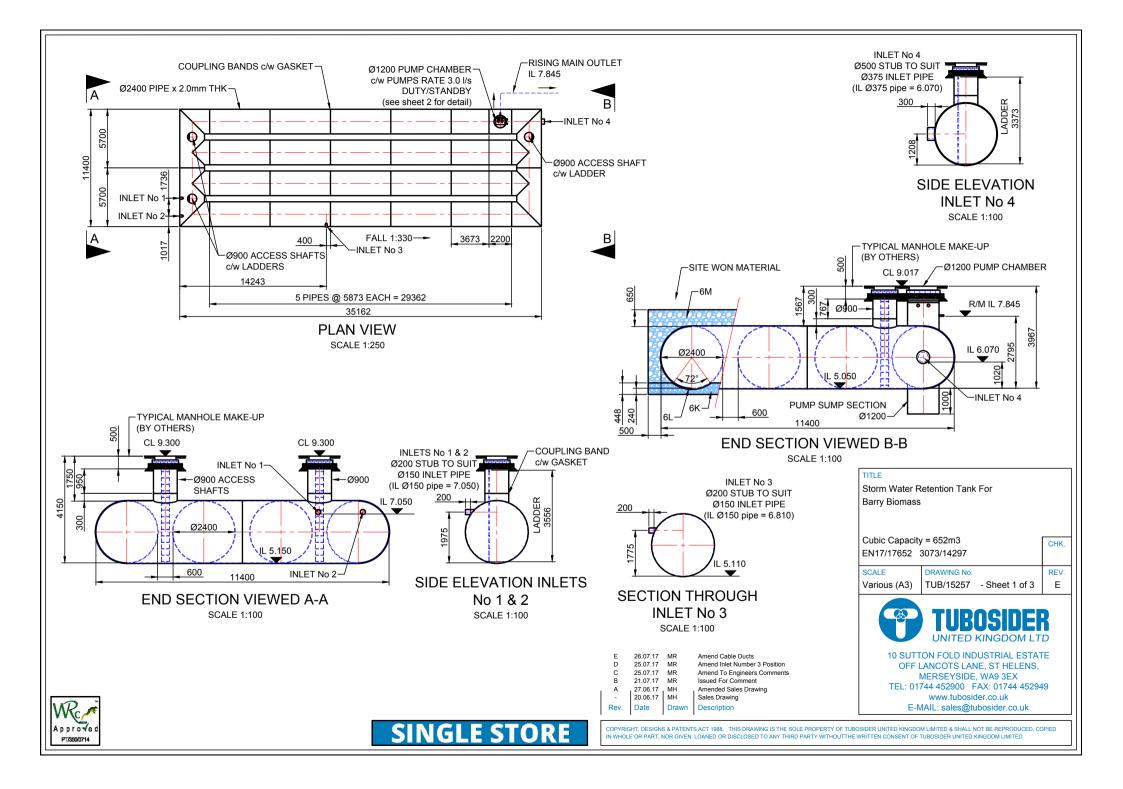
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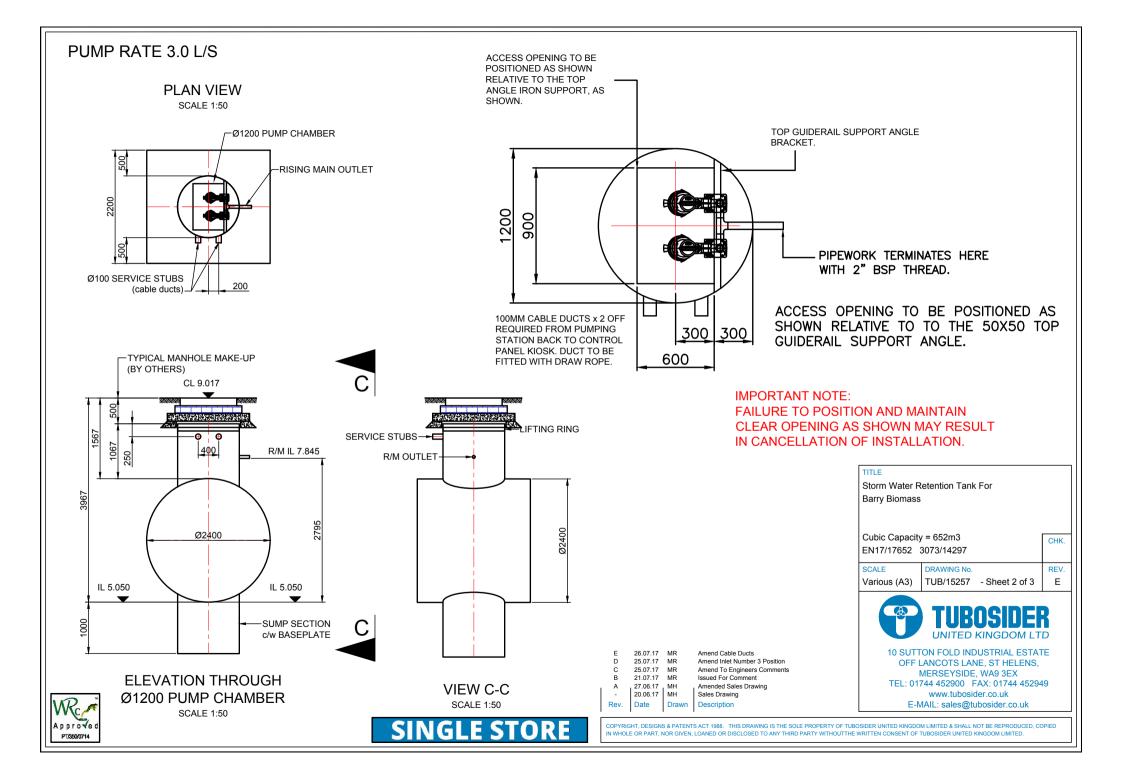
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BARRY 01 DWG 01 20132 - SITE SERVICES GA SHEET 2 OF 4
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BARRY_01_DWG_01_20136 - SITE SERVICES SCHEDULE









Backfill & Bedding Materials Specification

Lower bedding material	6K	Natural gravel, natural sand, crushed gravel, crushed rock other than argillaceous rock, crushed concrete, well - burn colliery spoil or any combination thereof.					
Upper bedding material 6L		Natural gravel, natural sand, crushed gravel, crushed rock other than argillaceous rock, crushed concrete, well - burnt colliery spoil or any combination thereof.					
Surround material	6M	Natural gravel, natural sand, crushed gravel, crushed rock other than argillaceous rock, crushed concrete, well - burnt colliery spoil or any combination thereof.					

PERCENTAGE BY MASS PASSING THE SIZE SHOWN

2

1.18

89 - 10060 - 10030 - 10015 - 100 5 - 70

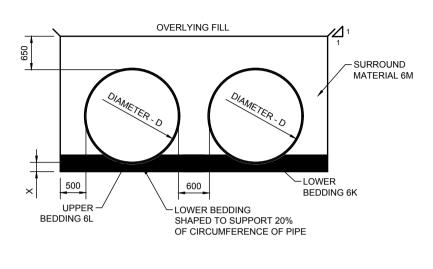
600

Size (mm) BS Series

5

10

100



EXCAVATION AND BACKFILL REQUIREMENTS Multiple Pipe Run

> Minimum depth (X) of Lower Bedding material to be equal to Diameter (D) divided by 10. Therefore for 1.8m pipe, X = 180mm etc.

The above information is drawn from the design manual tor roads and bridges. The materials listed below are as described in series 600 MCHW1.

TITLE									
Storm Water Retention Tank For									
Barry Biomass									
-									
Cubic Capacity	y = 652m3	CHK.							
EN17/17652	3073/14297								
SCALE DRAWING No. RE									
Various (A3)	Various (A3) TUB/15257 - Sheet 3 of 3								
	TUBOSIDE								
OFFI	ON FOLD INDUSTRIAL ESTAT ANCOTS LANE, ST HELENS, MERSEYSIDE, WA9 3EX 744 452900 FAX: 01744 45294 www.tubosider.co.uk	_							
E-N	/AIL: sales@tubosider.co.uk								



Material

Lower

bedding 6K

Upper

bedding 6L

Surround

6M

75

100

20

100



Size (um) BS Series

150

0 - 15

except

0-20 for crushed rock 63

0-10

0 - 10

26.07.17 MR

25.07.17 MR

25.07.17 MR

21.07.17 MR

27.06.17 , MH

20.06.17 MH

Date

Drawn

E D

С

в

Α

-Rev. Amend Cable Ducts

Issued For Comment

Sales Drawing

Description

Amended Sales Drawing

Amend Inlet Number 3 Position

Amend To Engineers Comments

300

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